



# Hydraulic conductivity, microstructure and texture of compacted claystone/ bentonite mixtures saturated with different solutions

M. Middelhoff, Olivier Cuisinier, Stéphane Gaboreau, F. Masrouri, J. Talandier, N. Michau

## ► To cite this version:

M. Middelhoff, Olivier Cuisinier, Stéphane Gaboreau, F. Masrouri, J. Talandier, et al.. Hydraulic conductivity, microstructure and texture of compacted claystone/ bentonite mixtures saturated with different solutions. *Applied Clay Science*, 2023, 241, pp.106982. 10.1016/j.clay.2023.106982 . hal-04119689

**HAL Id: hal-04119689**

**<https://hal.science/hal-04119689>**

Submitted on 6 Jun 2023

**HAL** is a multi-disciplinary open access archive for the deposit and dissemination of scientific research documents, whether they are published or not. The documents may come from teaching and research institutions in France or abroad, or from public or private research centers.

L'archive ouverte pluridisciplinaire **HAL**, est destinée au dépôt et à la diffusion de documents scientifiques de niveau recherche, publiés ou non, émanant des établissements d'enseignement et de recherche français ou étrangers, des laboratoires publics ou privés.

# Hydraulic conductivity, microstructure and texture of compacted claystone/ bentonite mixtures saturated with different solutions

M. Middelhoff <sup>1,2 †</sup>, O. Cuisinier <sup>1\*</sup>, S. Gaboreau <sup>3</sup>, F. Masrouri <sup>1</sup>, J. Talandier <sup>2</sup>, N. Michau <sup>2</sup>

<sup>1</sup> Université de Lorraine – LEMTA, UMR 7563 CNRS,  
(2 Rue du Doyen Marcel Roubault - BP 10162, 54000 Nancy CEDEX, France)

<sup>2</sup> Agence nationale pour la gestion des déchets radioactifs (Andra), France

<sup>3</sup> Bureau de recherches géologiques et minières (BRGM), France

[Olivier.Cuisinier@univ-lorraine.fr](mailto:Olivier.Cuisinier@univ-lorraine.fr)

Abstract: The French reference concept for the disposal of intermediate- and high-level nuclear waste in the Callovo-Oxfordian sedimentary rock formation (COX-claystone) considers the employment of crushed COX-claystone and its mixture with MX80-bentonite as potential backfill materials installed in drifts and shafts upon termination of the operational phase of the future repository. The fraction of MX80-bentonite in the mixture is limited to 30% in wet weight. Over time, the backfill will be saturated with solutions that originate in the surrounding rock formation and percolate through the concrete lining left in place. The main characteristic of the leachate will be its high pH-value. In addition to the swelling pressure, the sealing properties of the backfill are determined by the hydraulic conductivity. This laboratory experimental study aimed to understand the hydraulic conductivity evolution of potential backfill materials under realistic conditions in terms of compaction conditions, solution chemistry, temperature and hydraulic gradient, and to relate results determined at the macroscale to results of microstructural and textural analysis. Also, the impact of the solution chemistry was evaluated by these means. In the case of mixture-samples, no stabilization of hydraulic conductivity was detected, despite the duration of the experiments being about one year. There was no detectable impact of the solution chemistry in the course of experiments attributable to low reaction kinetics under imposed conditions. Subsequent microstructural analysis indicated material swelling triggering rearrangements in the pore space and lowering the fraction of hydraulic conductive voids. Interestingly, neither the saturation nor the solution chemistry had an impact on the texture of materials.

Keywords: Callovo-Oxfordian claystone, MX80-bentonite, Hydraulic conductivity, Microstructure, Texture, Solution chemistry

<sup>†</sup> currently contracted by: Gesellschaft für Anlagen- und Reaktorsicherheit (GRS) gGmbH, Theodor-Heuss-Straße 4, 38122 Braunschweig, Germany

## 1 Introduction and background

The French reference concept considers the disposal of intermediate and high level nuclear waste in the Callovo-Oxfordian sedimentary rock formation located 500 m below ground level (ANDRA, 2005). Henceforth, the sedimentary rock formation is referred to as Callovo-Oxfordian (COX) claystone. The encapsulated nuclear waste will be emplaced in horizontal drifts, which will be sealed and backfilled once the operational phase of the repository is terminated. Unlike the seals, backfill is envisaged to be installed in situ by means of conventional compaction techniques. It will swell upon its saturation and inhibit the flow of fluid phases from the emplacement drifts to accessible biosphere over time. Its installation particularly aims to limit the propagation of the excavation damaged/ disturbed zone (EDZ/ EdZ) and to close the hydraulic conductive pore space (ANDRA, 2005). In addition to the swelling pressure evolving under the constant volume conditions, the hydraulic conductivity can be considered as one of the key parameters which govern the sealing properties of backfill.

Potential backfill materials are re-employed COX-claystone and its mixture with MX80-bentonite. COX-claystone is obtained during drift excavation and processed afterwards. Henceforth, crushed and sieved COX-claystone is referred to as COX<sub>c</sub>. Its re-employment particularly aims to inhibit mineralogical and physico-chemical incompatibilities with the surrounding rock formation (ANDRA, 2005). A mixture composed of COX<sub>c</sub> and MX80-bentonite is considered alternatively, since the increased smectite fraction might enhance the barrier functions of backfill. Like other smectite-containing materials, COX<sub>c</sub> and its mixture might be affected by variations in the material characteristics (e.g. textural and microstructural characteristics, initial dry density, degree of saturation etc.), the environmental conditions (e.g. solution chemistry etc.) and stress history (e.g. preconsolidation pressure, overburden pressure etc.) with regard to their hydro-mechanical behavior (e.g. Alonso et al., 1987; Chen, 1988). It must be thus evaluated whether the sealing characteristics of backfill are stable to possible variations over time.

According to Burland (1990), the structure of smectite-containing materials comprises fabric as well as interparticle bonding. Even though fabric also provides information about the arrangement of particles, this study adopts texture as it describes the features of particles (e.g. shape, orientation towards each other) and pores (e.g. shape, volume) more precisely.

Numerous laboratory experimental studies addressed the impact of the saturating solution chemistry on the physico-chemical and textural characteristics (e.g. Ramírez et al., 2005; Herbert et al., 2008), the microstructure (e.g. Cuisinier et al., 2008; Fernández et al., 2014) and the hydro-mechanical behavior of smectite-containing materials (e.g. Villar, 2006; Karnland et al., 2007; Cuisinier et al., 2014; Zhang and Kröhn, 2019; Middelhoff et al., 2020). Considering the French reference concept, the impact of the pH is of major relevance, as described below.

Once the closure phase of the repository is terminated, the backfill might be saturated with solutions, which originate in the surrounding geological formation and percolate through the concrete lining left in place. The initial leachate is characterized by a pH above 13.5. This elevated pH is attributable to the release of potassium and sodium hydroxides from the concrete. After their leachate, the solution chemistry will be controlled by the dissolution of portlandite at a pH of 12.5 and later by the dissolution of cement phases at a pH ranging from 10.5 to 12.5. The phenomenon is referred to as hyperalkaline plume and might trigger different geochemical reactions while saturating the backfill (Huertas et al., 2000; Michau, 2005). Ion exchange at clay minerals can be expected once the alkaline solution percolates the backfill material. For instance, sodium ions can be replaced by calcium ions from the alkaline solution as their hydration potential is higher (Norrish, 1954). Simultaneously, the dissolution of clay minerals can be triggered as they remain relatively stable up to a pH of 9 and start dissolving once the pH exceeds 10. Secondary minerals, such as carbonates, clay minerals, cement phases, silicates, and zeolites, can precipitate or form afterwards. Their reaction kinetics and reaction mechanism are controlled by the pH of solutions, among other factors (Krauskopf, 1956). The analyses of

concrete lining/ claystone formation-interfaces sampled in situ indicated low reaction kinetics under real conditions (Gaboreau et al., 2011; Gaboreau et al., 2012). Thus, in order to determine a considerable impact of the solution chemistry on the backfill, the clay mineral/ pore water-system needs to remain thermodynamically unstable for a sufficiently long time.

To investigate the impact of the solution chemistry on the backfill materials in a reasonable period of time, most studies considered either simplified or extreme conditions, instead of realistic conditions. Simplified or extreme conditions comprise an increased liquid to solid ratio, an increased ionic strength of solutions, the employment of homoionic solutions and an increased temperature. In the case of hydraulic conductivity experiments, values can be presumably obtained faster by imposing higher hydraulic gradients. However, there is no information about the impact of the magnitude of the hydraulic gradient on the reaction kinetics.

The impact of saturating solution chemistry on the physico-chemical and textural characteristics of smectite-containing materials is mainly investigated by conducting batch experiments characterized by a high liquid to solid ratio (e.g. Herbert et al., 2004; Hofmann et al., 2004; Ramírez et al., 2005; Fernández et al., 2014; Elert et al., 2015). However, there are significantly different results, particularly regarding the cation exchange capacity and specific surface area. Differences are attributable to the differences in the composition, ionic strength, and pH of solutions and in imposed temperatures. In general, results lack transferability to compacted materials predominantly due to the high liquid to solid ratio.

Micro-, meso- and macropores characterize the structure of compacted smectite-containing materials in the unsaturated state and correspond to pores between clay particles, clay aggregates, and greater assemblies of clay aggregates and shielding grains, respectively. The existence of meso- and macropores is indicated by a bi-modal pore-size distribution (PSD) which is commonly determined by means of mercury intrusion porosimetry (MIP) experiments

(Alonso et al., 1987). In addition, N<sub>2</sub>-gas sorption experiments are performed to provide further information, in particular on the existence of some smaller mesopores and micropores.

There are only a few studies addressing the impact of solution chemistry, in particular its pH, on the microstructure of compacted smectite-containing materials. For instance, the study of Cuisinier et al. (2008) comprised an aging process, in which compacted clayey soil samples were saturated with a portlandite-saturated solution (pH > 12) under elevated temperatures (> 50°C). The aging process lasted longer than 12 months. Subsequent MIP-experiments revealed that the macropores increased significantly, whereas the mesopores remained stable. The dissolution of smectite accounted for the increase in the amount of macropores, and might in turn provoke an increase in hydraulic conductivity (Romero, 2013). In general, the results are less transferable to compacted backfill materials, since the elevated temperatures accelerated the reaction kinetics and might have induced different reaction mechanisms.

There is a wealth of studies aiming to analyze the impact of saturating solution chemistry on the hydraulic conductivity behavior of compacted backfill materials under realistic conditions (e.g. Mata, 2003; Villar, 2006; Ye et al., 2014; Zhang and Kröhn, 2019; Zhang, 2021). Realistic conditions refer to the initial compaction conditions of samples, the saturating solution chemistry, and the ambient temperature. Their major conclusion was that the impact vanishes upon the ionic strength decreases or the dry density increases. Results were attributed to the modification of the pore size distribution and to the mobility of the water molecules within the macropores. However, interpretations were mainly based on diffuse double layer (DDL) theories and thus lack direct experimental evidence.

Only a few studies related changes in the hydraulic conductivity behavior of smectite-containing materials to the impact of the saturating solution chemistry, in particular of the pH-value, on the physico-chemical and textural characteristics and microstructure (e.g. Pusch et al., 2003; Cuevas, 2005; Karnland et al., 2007; Cuisinier et al., 2014). Indeed, parts of the studies

of Pusch et al. (2003) and Cuevas (2005) were conducted under realistic conditions (e.g. compaction state, solution chemistry, ambient temperature) and are thus of special interest. However, both studies lacked completeness as they disregarded the microstructure and attributed changes in the hydraulic conductivity to the impact of the solution chemistry on the physico-chemical and textural characteristics only. Results indicated that the solution chemistry affected neither the hydraulic conductivity of materials nor their cation exchange capacity and specific surface area. Those observations were attributed to the low reaction kinetics. Despite the conclusion, the sealing properties of potential backfill materials were assumed to be long-term stable to the saturating solution chemistry under realistic conditions.

The literature review revealed that compared to the one-dimensional volume change behavior (e.g. compressibility, swelling pressure) the fluid transfer behavior of COX<sub>c</sub>-based materials was little investigated although it determines the sealing behavior of the backfill. Former experiment programs also disregarded relevant boundary conditions, in particular the pH-value of saturating solutions. This study thus aims to determine the hydraulic conductivity of COX<sub>c</sub> and its mixture with MX80-bentonite by means of constant-head experiments, whose conditions were believed to induce repository-like conditions in terms of compaction state, temperature, hydraulic gradient and solution chemistry. Subsequent N<sub>2</sub>-gas sorption and mercury intrusion porosimetry (MIP) experiments serve the purpose of linking differences in the hydraulic conductivity to changes in the texture and microstructure of materials. In particular, the saturation of materials with a hyperalkaline solution is expected to induce considerable changes. The analyse of the spatial distribution of textural and microstructural parameters allow to assess whether the hydraulic conductivity is still evolving in the short- and intermediate-term or reflects a long-term stable state. The experimental program involving the saturation of materials with a hyperalkaline solution and the approach of linking directly the evolution of hydraulic conductivity to textural and microstructural changes constitute the uniqueness of this study.

## 2 Materials

The materials investigated were COX<sub>c</sub> and its mixture with MX80-bentonite (Wyoming, USA). The mixture was composed of 70% COX<sub>c</sub> and 30% MX80-bentonite in wet weight. The limitation of MX80-bentonite to 30% was based on the reasons mentioned above (e.g. reductions of incompatibilities with the surrounding formation). COX-claystone was obtained during the excavation of drifts of Andra's URL in a depth of – 490 m (Bure, Meuse/ Haute-Marne region, France). Conil et al. (2018) highlighted that the mineralogical composition, water content and porosity of intact COX-claystone samples vary with their position in the rock formation. Correspondingly, COX<sub>c</sub> was expected to consist of about 46% phyllosilicates (e.g. illite, ordered illite/ smectite mixed layer minerals), 24% tectosilicates (e.g. quartz, feldspars), 27% carbonates (e.g. calcite, dolomite), and minor fractions of associated minerals (e.g. pyrite). In accordance with the reference concept, the subsequent processing of material comprised the crushing and sieving to a maximum grain size of 2 mm. The material was then stored in airtight containers. The wealth of information about the volume change, transfer, and sorption behavior of MX80-bentonite accounted for its selection to be mixed with COX<sub>c</sub>. MX80-bentonite was supplied by Laviosa-MPC SAS (Limay, France), who already processed the material to a maximum grain size of 2 mm before filling it in airtight containers. It is noteworthy that the supplier indeed sells its bentonite from Wyoming under the commercial name of Expangel SP7, but it is commonly identified under the name of MX80. Its mineralogical composition is dominated by phyllosilicates (e.g. smectites), whose fraction is about 80%. Other minerals are tectosilicates (e.g. quartz, feldspars) and carbonates (e.g. calcite) (e.g. Molinero-Guerra et al., 2020).

Information about the physical and physico-chemical characteristics of COX<sub>c</sub> and its mixture with bentonite are given in Table 1. Physical characteristics of COX<sub>c</sub> and its mixture with MX80-bentonite were taken from Middelhoff et al. (2020). The external specific surface area (SSA) and the cation exchange capacity (CEC) of COX<sub>c</sub> and the mixture were determined by

means of the BET- and cobalt hexamine methods (Orsini and Remy, 1976), respectively. The latter was also employed to determine the exchangeable cation species. Values of SSA and CEC of COX<sub>c</sub> were in good agreement with previous studies (Gaucher et al., 2004; Yven et al., 2007). The dominant exchangeable cation species in COX<sub>c</sub> were Ca<sup>2+</sup>, Mg<sup>2+</sup>, Na<sup>+</sup> and K<sup>+</sup>, listed according to their amount encountered. Corresponding amounts were 53%, 21%, 13% and 12% of adsorbed cations, respectively. The replacement of COX<sub>c</sub> by 30% MX80-bentonite led to a significant increase in the fraction of Na<sup>+</sup>-ions. The exchangeable cations were then dominated by Na<sup>+</sup>-ions, followed by Ca<sup>2+</sup>-, Mg<sup>2+</sup>- and K<sup>+</sup>-ions with respect to their amount. Corresponding amounts were 43%, 36%, 15% and 6% of adsorbed cations, respectively.

The grain size distribution curves of COX<sub>c</sub> and the mixture are shown in Figure 1. The size distribution of grains smaller than 0.8 mm was measured by means of laser diffractometry (ISO, 2009), whereas dry sieving was conducted in order to obtain the size distribution of the residual grains. As depicted, the distribution curves of both materials were comparable. The compaction behavior of both materials was investigated by means of standard and modified Proctor experiments. Materials were prepared at various water contents by adding corresponding quantities of deaired/ demineralized water. They were left in airtight containers for more than one week occasionally revolved to guarantee the homogeneous distribution of added water. A detailed description of results of standard and modified Proctor experiments and their interpretation are given in Middelhoff et al. (2020). Modified Proctor experiments revealed that the maximum dry density and optimum water content were 1.95 Mg/m<sup>3</sup> and 13.2%, respectively, in the case of COX<sub>c</sub>, and 1.72 Mg/m<sup>3</sup> and 18.2%, respectively, in the case of the mixture.

The laboratory experimental program comprised the saturation of both materials with three solutions differing in their composition and properties. Deaired/ demineralized water served as a reference solution. Henceforth, water referred to deaired/ demineralized water, and the subscripts SS and AS indicated the artificial site solution and the artificial alkaline solution,

respectively. The compositions of the site and alkaline solution corresponded to the rock formation water and to solutions believed to evolve from percolating through the concrete lining, respectively. Both solutions were prepared according to a protocol established and provided by Andra (Middelhoff et al., 2020). Their compositions and key properties are given in Table 2. The site and the alkaline solutions were comparable in terms of the order of magnitude of their ionic strength, however the alkaline solution was characterized by an elevated pH. The pH of AS corresponded to the solution chemistry controlled by the dissolution of portlandite.

### 3 Experiments

The hydraulic conductivity, microstructure and texture of COX<sub>c</sub> and its mixture with MX80-bentonite were determined by means of constant head experiments, mercury intrusion porosimetry (MIP) experiments and N<sub>2</sub>-gas sorption experiments, respectively. The sample preparation, experiment protocols adopted, and the program established are described below.

#### 3.1 Sample preparation

Materials were prepared according to the protocol presented in section 0. Samples were then compacted to the maximum dry density at the optimum water content by means of the static compaction method at a controlled deformation rate of 0.1 mm/s. The protocol envisaged the compaction of materials directly inside the mold in order to reduce the risk of the development of preferential paths along the sample/mold-interface. The initial height and diameter of all samples were 30 mm and 70 mm, respectively. Information about the initial characteristics of COX<sub>c</sub> and mixture-samples is given in Table 3.

In the case of the mixture-samples, the microstructural and textural analyses were performed shortly after the termination of the hydraulic conductivity experiments. Before they were analyzed, samples were cut perpendicularly to their center axis into discs of same height. The center axis corresponded to the imposed flow direction. The purpose of this preparation

step was to determine the possible spatial evolution of the microstructure and texture triggered by the saturation and the solution chemistry. Supplementary COX<sub>c</sub>- and mixture-samples were prepared in the same way to evaluate the impact of the partial material replacement and to compare the structure of material before and after saturation.

### 3.2 Hydraulic conductivity experiments

The setup of hydraulic conductivity experiments is depicted in Figure 2. The employed constant-volume hydraulic conductivity cell (rigid wall permeameter) was composed of its mold, two porous discs, its lid, and its bottom plate. The latter two included one o-ring each. The mold containing the sample was sandwiched between the porous discs. This assembly was then placed between the lid and the bottom part, which were finally screwed together.

The constant head method was considered to be most adequate to investigate the hydraulic conductivity of both samples (e.g. Dixon et al., 1999; Mata and Ledesma, 2003; Villar, 2006). By means of a constant volume/ pressure control unit, samples were saturated from the bottom upwards. Solutions left the cell at its top and were finally collected in a graduated flask. The accuracy of the measurement of outflow was about 0.5 mL. In order to avoid interaction processes of the solution with the atmosphere (e.g. evaporation, exchange of CO<sub>2</sub>), the flask was plugged, allowing only solutions to enter. The increase of gas pressure inside the flask was expected to be negligible.

Imposed injection pressures increased stepwise from 15 kPa to 90 kPa in the first weeks of the experiments. Considering a sample height of 30 mm, corresponding hydraulic gradients varied from 50 m/m to 300 m/m, which reflected the conditions encountered in situ. The pore pressure measured in undisturbed COX-claystone varies from 4,000 to 5,000 kPa (Seyedi et al., 2017). Assuming atmospheric pressure in the drift and a minimum extension of the EdZ of about 8 m, the corresponding hydraulic gradient varies from 50 m/m to 60 m/m. Hydraulic gradients that were imposed in this experiment program were still higher since there was a need

to accelerate the experiments. The minimum volume injected was equal to two times of the initial pore volume of the sample.

Hydraulic conductivity experiments were performed under an ambient temperature of 22°C. The temperature was in accordance with the results of Tourchi et al. (2021), who reproduced a full-scale in situ heating experiment in COX-claystone by performing numerical simulations. Their simulations revealed that the temperature at the edge of the access galleries would not exceed 23°C after waste emplacement.

### 3.3 Microstructural analysis

Mercury intrusion porosimetry (MIP) experiments were conducted by means of an Auto-Pore IV 9500 apparatus (Micromeritics, U.S.A.). Prior to the microstructural analysis, samples were outgassed under a residual pressure of  $1.6 \times 10^{-6}$  Pa at a temperature of 80°C for 24 hours. The residual pressure involves the condensation of pore water at lower temperatures and, in turn, negligible shrinkage. Gaboreau et al. (2016) evaluated different pre-treatment methods for microstructural analysis and showed that the method adopted yields coherent results.

The MIP-method bases on the principle of forcing a non-wetting fluid, like mercury, to enter a porous medium by incrementally increasing injection pressures. Detailed information about its principle and procedure are given in Cuisinier and Laloui (2004) and Delage et al. (2006), for instance. The result of MIP-experiments is the cumulative volume of mercury intruded as a function of the equivalent pore diameter. The pore size distribution (PSD) curve is deduced from those results in order to facilitate their interpretation (Juang and Holtz, 1986). It is defined as the derivative of the total volume of mercury intruded with respect to the common logarithm of the equivalent pore diameter. In this study, the approach of PSD-curve determination followed that presented by Juang and Holtz (1986) and used a constant value of  $\Delta(\log(d_i))$  being equal to 0.25. The determination of PSD by means of MIP is limited, regarding the intru-

sion of pores, whose diameter are smaller and larger than 7 nm and 600 nm, respectively. Results might also be impaired through the occurrence of the bottleneck phenomenon or pore entrapment, for instance (e.g. Romero et al., 1999; Delage et al., 2006; Romero, 2013).

### 3.4 Textural analysis

Nitrogen gas ( $N_2$ ) adsorption-desorption cycles were determined by means of an ASAP 2050 volumetric adsorption analyzer (Micromeritics, U.S.A.) at 77K. Block samples were initially outgassed under a residual pressure of  $1.6 \times 10^{-6}$  Pa at 80°C. The outgassing lasted 24 hours. The nitrogen gas adsorption method adopts the physical adsorption of non-reactive gas molecules, like nitrogen, on solid external surfaces. Information about its principle and procedure is given in Gregg and Sing (1982), for instance. BET theory and a generalized  $t$ -plot method were adopted to describe the texture of solids (Lippens and Boer, 1965).

### 3.5 Pore classification

The diameter of macropores is larger than  $5 \times 10^1$  nm, whereas the diameter of mesopores ranges between  $2 \times 10^0$  nm and  $5 \times 10^1$  nm. Micropores are characterized by a diameter smaller than  $2 \times 10^0$  nm (e.g. Thommes et al., 2015). In this study, the lower limit of the class of mesopores was shifted to the entrance pore diameter of  $7 \times 10^0$  nm, since the mesopores, whose diameters are smaller than  $7 \times 10^0$  nm, cannot be detected by means of the MIP method. The range of the class of mesopores was accordingly reduced by 10% and the bias was expected to be negligible in the further interpretation of results.

### 3.6 Experimental program

The program comprised four samples of  $COX_c$  and four samples of the mixture, respectively indicated by the prefix C and M in their sample identification. The suffix I, W, SS and AS referred to samples analyzed in their initial state, and to samples analyzed during/ after the

saturation with water, site solution and alkaline solution, respectively. The detailed experiment program is given in Table 4. In terms of structural alterations, M-SS and M-AS were expected to react most considerably to the saturation with different solutions due to their elevated smectite fraction and particular solution chemistry. Therefore, they were selected for the subsequent microstructural and textural analysis. This approach provided the basis to evaluate the impact of the saturation process and saturating solution chemistry on the material properties.

## 4 Results

In this section, the results obtained in the hydraulic conductivity experiments are presented first, followed by the results of subsequent microstructural and textural analysis. The presentation reflects the sequence of data acquisition.

### 4.1 Hydraulic conductivity experiments

Values of hydraulic conductivity were determined upon the establishment of steady state condition (hydraulic equilibrium) that was indicated by stabilized rates of inflow and outflow of solutions. Moreover, the hydraulic equilibrium implied full saturation of samples. Both materials were saturated with water, the site and alkaline solution. The normalized volume of inflow and the hydraulic conductivity of  $COX_c$  are plotted against the normalized volume of outflow in Figure 3a and c, respectively, whereas Figure 3b and d depict the evolution of the normalized volume of inflow and hydraulic conductivity of the mixture as a function of the normalized volume of outflow. The normalized volume of inflow and outflow are the ratios of volume of measured inflow and outflow, respectively, to the initial pore volume and serves the purpose of emphasizing the amount of pore solution exchanged. Data obtained were subsequently processed by means of a moving average method.

Focusing on  $COX_c$ , inflow rates increased once the hydraulic gradient was raised stepwise and stabilized. The inflow volume ranged from the threefold to the fivefold of the initial pore

volume. The greatest volume of solution was injected into C-AS. In all three experiments, the onset of outflow was determined once the volume of solution corresponding to one initial pore volume was injected into the samples. Shortly after the onset of outflow was detected, steady state conditions established at an average flow rate ranging from 2 mL/day to 2.5 mL/day. The former rate refers to C-W. As depicted in Figure 3a and c, pore solution was exchanged at least twice within a period of about 60 days. The smallest number of exchanges was exhibited by C-W, which corresponded to the smaller average flow rates. In the course of the experiments, values of hydraulic conductivity calculated decreased initially and stabilized at values between  $6 \times 10^{-11}$  m/s to  $9 \times 10^{-11}$  m/s. Samples C-SS and C-AS exhibited slightly higher hydraulic conductivities than C-W.

Total inflow volume into mixture-samples varied from the twofold to the threefold of the initial pore volume and were thus less than that injected into COX<sub>c</sub>-samples, despite the fact that the experiments lasted five to six times longer (300 days – 360 days). In the case of M-W and M-SS, outflow was detected after injecting the volume of solution corresponding to one initial pore volume. In the case of M-AS, the volume of solution corresponding to two initial pore volumes were injected prior to the onset of outflow that might be attributable to a greater amount of air in the porous disc of the bottom plate. Indeed, steady state conditions were established shortly after detecting an outflow. However, the inflow and outflow rates decreased steadily in the course of the experiments, starting from 0.4 mL/day at the beginning to 0.2 mL/day in the end. The trend is also identifiable in Figure 3d. Values of hydraulic conductivity accordingly decreased from  $2 \times 10^{-11}$  m/s to  $3 \times 10^{-12}$  m/s. Comparing the values of hydraulic conductivity at the same normalized outflow of 0.6, M-AS exhibited the smallest values.

The hydraulic conductivity of COX<sub>c</sub> was by one order of magnitude higher than that of the mixture. The difference became even more considerable as the values of COX<sub>c</sub> stabilized, whereas the values of the mixture progressively decreased in the course of the experiment.

Apart from that, the chemistry of the site and alkaline solution had no considerable impact on the hydraulic conductivity of both materials. The characterization of the microstructure and texture was of special interest at this stage, in order to evaluate whether the evolution at macroscale was reflected in the material properties at the nano- and microscale.

## 4.2 Microstructural analysis

Samples were prepared by cutting them perpendicularly to their center axis into two discs of same height (15 mm) in the case of MIP experiments. Henceforth, top part (T) and bottom part (B) referred to the upper part and lower part of the initial sample, respectively. Results of MIP-experiments performed on C-I, M-I, M-SS and M-AS are depicted in Figure 4.

In general, the quantities of micropores, mesopores and macropores are expressed as micro-void ratio ( $e_\mu$ ), meso-void ratio ( $e_m$ ) and macro-void ratio ( $e_M$ ), respectively. The cumulative void ratio was calculated as the ratio of the cumulated volume of mercury intruded in voids to the volume of solids. The micro-void ratio was defined as the difference between the total void ratio ( $e_{Tot}$ ) and the sum of the meso-void ratio and the macro-void ratio. Since the porosity ( $n$ ) of top and bottom parts of each sample was also measured in MIP-experiments, the total void ratio ( $e_{Tot}$ ) was calculated as the ratio of porosity to the difference of 1 to porosity. The approach was exemplified by the top parts of samples in Figure 4. Complementing Figure 4, Table 5 provides absolute and relative values of micro-, meso-, and macro-void ratios. It facilitates the evaluation of the impact of partial replacement of COX<sub>c</sub> by MX80-bentonite, saturation and solution chemistry. Relative values were defined as the ratios of micro-, meso-, and macro-void ratio to the total void ratio of the corresponding sample part (= normalization) and are provided in order to disregard the impact of initial dry density, in particular on the macro-void ratios.

In the case of C-I, the volume of pores appeared to reduce along the center axis of the sample since the cumulative volume of mercury intruded in C-I (T) was higher than that of C-I (B). In the case of M-I, the opposite was observed. A distinct double peak was revealed by

the pore size distribution curves in the upper parts of Figure 4. It indicated the existence of meso- and macropores. As depicted in Figure 4a, the peaks of meso- and macropores of C-I (T) and C-I (B) occurred at the same pore diameter of about  $4 \times 10^1$  nm and  $2.5 \times 10^4$  nm, respectively. The comparison of relative meso- and macro-void ratios of C-I (T) to those of C-I (B) revealed similar values. M-I (T) was characterized by meso- and macropores at pore diameters of about  $3 \times 10^1$  nm and  $5 \times 10^4$  nm, respectively (Figure 4c). Meso- and macropores of M-I (B) occurred at pore diameters of about  $4 \times 10^1$  nm and  $2.5 \times 10^4$  nm, respectively. The relative meso-void ratio of M-I (T) was 5% higher than that of M-I (B), whereas the relative macro-void ratio of M-I (T) was 8% lower than that of M-I (B).

As highlighted in the lower parts of Figure 4 and in Table 5, the microstructure of M-SS and M-AS evolved comparably, despite they were saturated with different solutions. Top parts of M-SS and M-AS were characterized by the majority of mesopores occurring at a diameter of  $2.5 \times 10^1$  nm. Compared to those of M-I (T), relative meso-void ratios of M-SS (T) and M-AS (T) increased by 7% and 9%, respectively. The majority of macropores was determined at a diameter of  $4 \times 10^4$  nm in the cases of M-I (T) and M-AS (T), and at a diameter of  $2 \times 10^4$  nm in the case of M-SS (T). The comparison of the relative macro-void ratio of M-I (T) to those of M-SS (T) and M-AS (T) revealed similar values. M-AS (B) was characterized by a mesopore diameter of  $2 \times 10^1$  nm, whereas the mesopores of M-SS (B) occurred at  $4 \times 10^1$  nm. Relative meso-void ratios of both M-SS (B) and M-AS (B) were increased by about 8% and 15%, respectively, when they are compared to M-I (B). Bottom parts of mixture samples were characterized by the majority of macropores occurring at a diameter of  $2 \times 10^4$  nm. Interestingly, an additional macropore population was determined at diameters of  $8 \times 10^4$  nm in the case of M-SS (B). Compared to those of M-I (B), relative macro-void ratios of M-SS (B) and M-AS (B) reduced by about 9% and 6%, respectively.

The similar relative micro-, meso- and macro-void ratios of C-I and M-I accounted for the fact that the impact of the partial replacement of COX<sub>c</sub> by MX80-bentonite remained unclear yet. Unlike the solution chemistry, the saturation had an impact on the microstructure of samples since the relative micro- and macro-void ratios decreased and aligned with each other, and the relative meso-void ratios increased.

### 4.3 Textural analysis

In the case of textural analysis, sample preparation involved to cut samples perpendicularly to their center axis into three discs of same height (10 mm) in order to identify possible spatial variations in the N<sub>2</sub>-gas sorption isotherms and the external specific surface area (SSA) along the flow direction. Henceforth, top part (Top), middle part (Middle) and bottom part (Bottom) referred to the upper, middle and lower parts of the initial sample, respectively. Sorption isotherms determined by means of the N<sub>2</sub>-gas sorption method are depicted in Figure 5.

Isotherms displayed a Type II behavior and a H3 hysteresis loop. The reversible Type II isotherm is the normal form of isotherm obtained with a macroporous adsorbent. The Type II represents unrestricted multilayer adsorption build up and the occurrence of mesopores. The Type H3 loop not exhibiting any limited adsorption at high P/P<sub>0</sub>-ratios was coherent with aggregates of plate-like particles. It gave rise to slit-shaped pores and proved that the samples were mesoporous. Isotherms gave linear BET plots from 0.04 to 0.4 P/P<sub>0</sub> for all tested samples.

SSA-values obtained by means of BET- and *t*-plot-method are compiled in Table 6. Calculated BET specific surface areas (*S*<sub>BET</sub>) and associated monolayer capacity (*V*<sub>*m*</sub>) exhibited neither variations in SSA among mixture-samples, nor an evolution of SSA over the length of the individual samples. *t*-plot results were coherent with the *S*<sub>BET</sub>- values.

## 5 Discussion

The study aimed to understand the evolution of hydraulic conductivity of COX<sub>c</sub> and its mixture with MX80-bentonite under quasi-realistic conditions by relating the results obtained at the macroscale to those obtained at the microscale. Quasi-realistic conditions refer to the fact that the compaction conditions, solution chemistry, ambient temperature and hydraulic gradient were believed to be in a realistic range. Since the saturation itself as well as the saturation with solutions of different chemistry affected the materials in more or less pronounced manner, it appeared to be suitable to arrange the discussion accordingly.

### 5.1 Impact of saturation

The existence of meso- and macropores in COX<sub>c</sub> and its mixture with MX80-bentonite was indicated by bi-modal PSD-curves. This finding was consistent with the literature as smectite-containing materials are usually characterized by such a double-structure if they are compacted at the dry side of optimum water content (Alonso et al., 1987).

Highlighted in Table 5, the void ratio of M-I (T) was lower than that of M-I (B). The density gradient was induced by the static compaction method and ensued as a shear stress evolved at the interface between the material and mold (e.g. Villar, 2006). The decrease in initial dry density raised the total pore volume, in particular the macropore volume. This finding was consistent with the literature (e.g. Lloret and Villar, 2007). Interestingly, the static compaction method had no impact on the distribution of pores in C-I (T) and C-I (B), although its water content was smaller than that of M-I. As a consequence, the evolution of higher shear stresses and a density gradient was expected.

The normalization of void ratios allowed to disregard the impact of densification on the microstructure of C-I and M-I and to evaluate quantitatively the impact of the partial replacement of COX<sub>c</sub> by MX80-bentonite. The comparison of C-I to M-I indicated that M-I indeed incorporated more macro-pores but less meso-pores on average. The average micro-void ratios

of C-I and M-I were comparable (Table 5). Apparently, the higher macro-void ratio of M-I ensued as the higher amount of smectite particles provoked the formation of more card-house-like aggregates, including more inter-aggregate pores.

The saturation of M-SS and M-AS led to a decrease in macro-void ratios and to an increase in micro- and meso-void ratios. Small differences along the center axis likely indicated a homogeneous saturation of samples. The decrease in macro-void ratio was attributable to the closure of hydraulic conductive pores ensuing as smectite particles and aggregates swell and rearrange upon saturation (e.g. Komine, 2004, 2010).

Information about COX<sub>c</sub> and its mixtures with MX80-bentonite, in particular their one-dimensional compression and swelling behavior under saturation, is given by Tang et al. (2010), Wang et al. (2012), Zeng et al. (2019), Middelhoff et al. (2020) and Robinet et al. (2021), for instance. In contrast, their hydraulic conductivity behavior under saturation was less intensely analyzed. The studies of Zhang and Kröhn (2019) and Zhang (2021) were thus of major interest since they determined the saturated hydraulic conductivity of compacted COX<sub>c</sub> and different COX<sub>c</sub>/MX80-bentonite-mixtures, among other hydro-mechanical parameters. Prior to testing, all samples of Zhang and Kröhn (2019) were prepared to target dry densities of about 2.0 Mg/m<sup>3</sup> at initial water contents of about 4%. The corresponding total void ratios were about 0.33. In the case of COX<sub>c</sub>, values of hydraulic conductivity were about  $5 \times 10^{-13}$  m/s and assumed to reflect a long-term stable state. Tang et al. (2010) also focused on the saturated hydraulic conductivity of COX<sub>c</sub>, whose initial water contents ranged between 2% to 8%. Target dry densities were also about 2.0 Mg/m<sup>3</sup>. They determined a saturated hydraulic conductivity of about  $3 \times 10^{-12}$  m/s. Moreover, their results indicated to a negligible impact of the initial water content on the hydraulic conductivity. The one and two orders of magnitude lower saturated hydraulic conductivities of Tang et al. (2010) and Zhang and Kröhn (2019) were attributable to the lower initial dry densities and/ or higher initial total void ratios of C-SS and C-AS most probably

(Table 5). In the case of a mixture that was composed of 80% COX<sub>c</sub> and 20% MX80-bentonite in weight, Zhang and Kröhn (2019) measured values of saturated hydraulic conductivities varying from  $1 \times 10^{-12}$  m/s to  $2 \times 10^{-12}$  m/s. Their values were in good agreement with the values obtained for M-SS and M-AS. Considering the initial compaction conditions, the recent results were remarkable since Zhang and Kröhn (2019) and Zhang (2021) compacted mixture samples to higher initial dry densities at lower initial water content with higher compaction energy before they performed hydraulic conductivity experiments. Nevertheless, the magnitude of saturated hydraulic conductivities was the same ( $\times 10^{-12}$  m/s). This comparison highlighted the efficiency of the rearrangement of hydraulic conductive macropores ensuing as the smectite fraction swells upon its saturation. Further, it accounted for the difference in the performance of COX<sub>c</sub> and its mixture with MX80-bentonite.

As the values of hydraulic conductivity progressively decreased, it was hypothesized that the microstructure in COX<sub>c</sub>/MX80-bentonite-mixture samples was still rearranging after more than one year of saturation, and that values of hydraulic conductivities were likely reflecting a short- or intermediate-term state. In order to corroborate the hypothesis, the kinetics of hydraulic conductivity experiments were compared to that of constant-volume swelling pressure experiments of Middelhoff et al. (2020). They performed those experiments on similar samples under comparable conditions and observed the establishment of constant swelling pressure after seven days of saturation. The different kinetics was attributed to the hydraulic gradient exceeding 500 m/m in constant-volume swelling pressure experiments. The higher hydraulic gradient involved higher rates of normalized inflow, thus a greater volume of solution for hydro-mechanical processes was accessible in less time. This might account for an accelerated kinetics in constant-volume swelling pressure experiments. Nevertheless, it is recommendable to per-

form combined constant-volume swelling pressure/ constant-head hydraulic conductivity experiments in order to better understand the kinetics of swelling pressure build-up and swelling-induced pore closure and their coupling.

## 5.2 Impact of solution chemistry

The texture and microstructure of M-SS and M-AS were more affected by the saturation than by the solution chemistry. The solution chemistry, in particular that of the alkaline solution, had no impact neither on the shape of N<sub>2</sub>-gas sorption isotherms nor on SSA-values nor on their spatial distribution (e.g. Figure 5). Considerable textural and microstructural changes were expected as the alkaline solution percolates the materials over time and dissolve the clay minerals therein. In turn, their dissolution alters the shape of mesopores and increases the fraction of macropores (Karnland et al., 2007; Cuisinier et al., 2008; Cuisinier et al., 2014). The impact of the solution chemistry must be considered to be highly time-dependent. The reaction kinetics appeared to be low under realistic conditions, and dissolution processes presumably did not come into effect, yet.

There was no considerable impact of the solution chemistry on the hydraulic conductivity behavior exhibited by M-W and M-SS. It might be attributable to the low ionic strength of the site solution ( $I_{SS} = 105$  mmol/L). According to the theories of diffuse double layers (DDL), the solution chemistry has an impact on the hydraulic conductivity behavior of smectite-based materials, when diffuse double layers evolve in macropores and immobilize water molecules therein. The ion concentration in solution is inversely proportionally related to their layer thickness. An increase in hydraulic conductivity ensues as the ion concentration increases and, in turn, diffuse double layers diminish (e.g. Dixon, 2000; Villar, 2006; Castellanos et al., 2008).

In contrast, M-AS exhibited lowest values of hydraulic conductivity at the same normalized outflow of 0.6. This might be attributable to the clogging of hydraulic conductive macropores through swelling as well as precipitation of Ca(OH)<sub>2</sub>. The majority of pores in

M-AS were initially filled with water. Upon saturation, the pH-value of the alkaline solution dropped as it mixed with the pore water, and the consequent precipitation of  $\text{Ca}(\text{OH})_2$  intensified the clogging of macropores.  $\text{Ca}(\text{OH})_2$  precipitated until the alkaline solution determined the pH-value of the pore solution, and dissolved then again. Since the precipitation of  $\text{Ca}(\text{OH})_2$  changes the texture of materials, in particular their SSA-values, but  $\text{N}_2$ -gas sorption analyses revealed no changes, it can be hypothesized that  $\text{Ca}(\text{OH})_2$  already dissolved at that stage. An analyzes of the effluent and comparison of its  $\text{Ca}(\text{OH})_2$ -content to that of the influent can corroborate the hypothesis. Considering Middelhoff et al. (2020), the precipitation of  $\text{Ca}(\text{OH})_2$  presumably has no impact on the evolution of swelling pressure, unlike on that of hydraulic conductivity.

Apart from the low reaction kinetics under imposed conditions, the low volume of pore solution exchanged might account for the apparently negligible impact of the solution chemistry. As shown in Figure 3b, pore solution was exchanged less than once. Geochemical reactions, in particular dissolution and precipitation processes, appeared to not come into effect yet, despite the experiment duration of more than 300 days. The adoption of a different termination criteria is thus recommended that additionally considers the composition, ionic strength, and electric conductivity of the influent and effluent in order to predict the initiation of geochemical reactions in the course of the experiment more precisely. For instance, Jo et al. (2005) carried out long-term hydraulic conductivity experiments on geosynthetic clay liners. Values of hydraulic conductivity were measured once hydraulic equilibrium established and once both hydraulic and chemical equilibrium established. Unlike the former, the latter criteria was not met until the pore solution was exchanged several times. The comparison of results revealed an underestimation of the hydraulic conductivity by a factor of two when the termination criteria relied on the hydraulic equilibrium criteria exclusively. This also highlights the importance of volume of pore solution exchanged.

## 6 Conclusions

Crushed Callovo-Oxfordian claystone (COX<sub>c</sub>) and a mixture composed of 70% COX<sub>c</sub> and 30% MX80-bentonite in weight might be employed to backfill drifts and shafts of a future nuclear waste repository located in deep sedimentary rock formations. In addition to the swelling pressure, the hydraulic conductivity is one of the key parameters determining the performance of the backfill. This study aimed to understand the evolution of the hydraulic conductivity by means of laboratory experiments, whose boundary conditions were believed to portray the real conditions in terms of compaction conditions, solution chemistry, temperature and hydraulic gradient. Subsequently conducted microstructural and textural analysis allowed to relate the evolution of the hydraulic conductivity at the macroscale to changes at the microscale possibly affected by the solution chemistry, in particular the pH-value. Following major conclusions can be drawn for COX<sub>c</sub> and its mixture with MX80-bentonite:

1. The microstructural analysis showed considerable microstructural rearrangements, which were related to closure of macropores triggered by material swelling upon saturation. The interpretation was consistent with the observations and the literature. The negligible impact of the solution chemistry was attributed to the low volume of exchanged pore solution under imposed conditions.
2. In the case of COX<sub>c</sub> and the mixture, values of hydraulic conductivity were in good agreement with the literature. COX<sub>c</sub> exhibited stabilized values of hydraulic conductivity soon upon establishing steady state conditions. Conversely, values of hydraulic conductivity of the mixture progressively decreased in the course of the experiment. Considering the results of microstructural analysis, the closure of hydraulic conductive macropores through material swelling upon saturation likely accounted for the evolution of the hydraulic conductivity on the one side and for the difference in the performance of COX<sub>c</sub> and the mixture on the other side. The duration of some experiments was longer than one

year and highlighted the low kinetics of geochemical reactions under the imposed conditions. Hence, the hydraulic conductivity appeared to be little affected by the solution chemistry within one year of experiment duration. In order to study the time-dependent impact of solution chemistry more precisely, it is of major interest to conduct experiments which involve the exchange of several volumes of pore solution.

This study revealed that the replacement of COX<sub>c</sub> by 30% MX80-bentonite in weight enhanced considerably the barrier functions of compacted backfill materials under realistic conditions in terms of their short- and intermediate-term hydraulic conductivity. The enhancement can be attributed to the more effective clogging of hydraulic conductive voids. However, important questions arose while evaluating the experiments. For instance, it is still unclear how the hydraulic conductivity evolves under imposed conditions in long-term, particularly with regard to the time-dependent impact of the solution chemistry that must not be considered as negligible and of the consolidation of the backfill material induced by the convergence of the rock formation.

## Declaration of interest

The authors wish to confirm that there are no known conflicts of interest associated with this publication and there has been no significant financial support for this work that could have influenced its outcome.

## Funding

The work presented in this paper has been carried out during the preparation of the PhD thesis of the first author, which is funded by Andra (France), the French agency in charge of the management and disposal of nuclear waste.

## Acknowledgement

The authors thank Dr. F. Claret of BRGM and Dr. G. Armand of Andra for fruitful discussions that helped to improve this article. Moreover, the authors gratefully thank the two anonymous reviewers for their rigorous revisions.

## 7 References

- Alonso, E.E., Gens, A., Whight, D.W., 1987. General Report, in: Groundwater effects in geotechnical engineering. Ninth european conference on soil mechanics and foundation engineering, Dublin. 31.08. - 03.09. A. A. Balkema, Rotterdam, Brookfield, pp. 1087–1146.
- ANDRA, 2005. Dossier 2005 Argile Synthesis: Evaluation of the feasibility of a geological repository in an argillaceous formation. Meuse/ Haute-Marne site. Collection les Rapports. Agence nationale pour la gestion des déchets radioactifs, Châtenay-Malabry, 1497 pp.
- Burland, J.B., 1990. On the compressibility and shear strength of natural clays. *Géotechnique* 40, 329–378.
- Castellanos, E., Villar, M.V., Romero, E., Lloret, A., Gens, A., 2008. Chemical impact on the hydro-mechanical behaviour of high-density FEBEX bentonite. *Physics and Chemistry of the Earth, Parts A/B/C* 33, S516-S526.
- Chen, F.H., 1988. Foundations on expansive soils. Elsevier science pub, Amsterdam, New York, 1463 pp.
- Conil, N., Talandier, J., Djizanne, H., La Vaissière, R. de, Righini-Waz, C., Auvray, C., Morlot, C., Armand, G., 2018. How rock samples can be representative of in situ condition: A case study of Callovo-Oxfordian claystones. *Journal of Rock Mechanics and Geotechnical Engineering* 10, 613–623.
- Cuevas, J., 2005. Geochemical reactions in FEBEX bentonite. In: Michau, N. (Ed.) *ECOCLAY II - Effects of cement on clay barrier performance. Phase II. Final report*, pp. 105–117.

617 Cuisinier, O., Deneele, D., Masrouri, F., Abdallah, A., Conil, N., 2014. Impact of high-pH  
618 fluid circulation on long term hydromechanical behaviour and microstructure of com-  
619 pacted clay from the laboratory of Meuse-Haute Marne (France). *Applied Clay Science*  
620 88-89, 1–9.

621 Cuisinier, O., Laloui, L., 2004. Fabric evolution during hydromechanical loading of a com-  
622 pacted silt. *International Journal for Numerical and Analytical Methods in Geomechanics*  
623 28, 483–499.

624 Cuisinier, O., Masrouri, F., Pelletier, M., Villieras, F., Mosser-Ruck, R., 2008. Microstructure  
625 of a compacted soil submitted to an alkaline PLUME. *Applied Clay Science* 40, 159–170.

626 Delage, P., Marcial, D., Cui, Y.J., Ruiz, X., 2006. Ageing effects in a compacted bentonite: a  
627 microstructure approach. *Géotechnique* 56, 291–304.

628 Dixon, D.A., 2000. Porewater salinity and the development of swelling pressure in bentonite-  
629 based buffer and backfill materials 2000-04. POSIVA Oy, Helsinki, 51 pp. Accessed 27  
630 October 2019.

631 Dixon, D.A., Graham, J., Gray, M.N., 1999. Hydraulic conductivity of clays in confined tests  
632 under low hydraulic gradients. *Canadian Geotechnical Journal* 36, 815–825.

633 Elert, K., Pardo, E., Rodriguez-Navarro, C., 2015. Mineralogical Evolution of Di- and Trioc-  
634 tahedral Smectites in Highly Alkaline Environments. *Clays and Clay Minerals* 63, 414–  
635 431.

636 Fernández, R., Ruiz, A.I., Cuevas, J., 2014. The role of smectite composition on the hyper-  
637 alkaline alteration of bentonite. *Applied Clay Science* 95, 83–94.

638 Gaboreau, S., Lerouge, C., Dewonck, S., Linard, Y., Bourbon, X., Fialips, C.I., Mazurier, A.,  
639 Prêt, D., Borschneck, D., Montouillout, V., Gaucher, E., Claret, F., 2012. In-situ interac-  
640 tion of cement paste and shotcrete with claystones in a deep disposal context. *American*  
641 *Journal of Science* 312, 314–356.

642 Gaboreau, S., Prêt, D., Tinseau, E., Claret, F., Pellegrini, D., Stammose, D., 2011. 15 years of  
643 in situ cement–argillite interaction from Tournemire URL: Characterisation of the multi-  
644 scale spatial heterogeneities of pore space evolution. *Applied Geochemistry* 26, 2159–  
645 2171.

646 Gaboreau, S., Robinet, J.-C., Prêt, D., 2016. Optimization of pore-network characterization of  
647 a compacted clay material by TEM and FIB/SEM imaging. *Microporous and Mesoporous*  
648 *Materials* 224, 116–128.

649 Gaucher, E., Robelin, C., Matray, J.M., Négrel, G., Gros, Y., Heitz, J., Vinsot, A., Rebours,  
650 H., Cassagnabère, A., Bouchet, A., 2004. ANDRA underground research laboratory: inter-  
651 pretation of the mineralogical and geochemical data acquired in the Callovian–Oxfordian  
652 formation by investigative drilling. *Physics and Chemistry of the Earth, Parts A/B/C* 29,  
653 55–77.

654 Gregg, S.J., Sing, K.S., 1982. Adsorption, surface area and porosity, 2nd ed. Academic Press,  
655 London, New York.

656 Herbert, H.-J., Kasbohm, J., Moog, H., Henning, K.-H., 2004. Long-term behaviour of the  
657 Wyoming bentonite MX-80 in high saline solutions. *Applied Clay Science* 26, 275–291.

658 Herbert, H.-J., Kasbohm, J., Sprenger, H., Fernández, A.M., Reichelt, C., 2008. Swelling  
659 pressures of MX-80 bentonite in solutions of different ionic strength. *Physics and Chemis-*  
660 *try of the Earth, Parts A/B/C* 33, S327-S342.

661 Hofmann, H., Bauer, A., Warr, L.N., 2004. Behavior of Smectite In Strong Salt Brines Under  
662 Conditions Relevant to the Disposal of Low- to Medium-grade Nuclear Waste. *Clays and*  
663 *Clay Minerals* 52, 14–24.

664 Huertas, F., Farias, J., Griffault, L., Leguey, S., Cuevas, J., Ramírez, S., Vigil de la Villa, R.,  
665 Cobena, J., Andrade, C., Alonso, M.C., Hidalgo, A., Parneix, J.C., Rassineux, F., Bouchet,

666 A., Meunier, A., Decarreau, S., Petit, S., Vieillard, P., 2000. ECOCLAY I - Effects of ce-  
 667 ment on clay barrier performance. Final report. Off. for Off. Publ. of the Europ. Commu-  
 668 nities, Luxembourg, 140 pp.

669 ISO, 2009. Particle size analysis: Laser diffraction methods.

670 Jo, H.Y., Benson, C.H., Shackelford, C.D., Lee, J.-M., Edil, T.B., 2005. Long-Term Hydrau-  
 671 lic Conductivity of a Geosynthetic Clay Liner Permeated with Inorganic Salt Solutions. J.  
 672 Geotech. Geoenviron. Eng. 131, 405–417.

673 Juang, C.H., Holtz, R.D., 1986. A probabilistic permeability model and the pore size density  
 674 function. International Journal for Numerical and Analytical Methods in Geomechanics  
 675 10, 543–553.

676 Karnland, O., Olsson, S., Nilsson, U., Sellin, P., 2007. Experimentally determined swelling  
 677 pressures and geochemical interactions of compacted Wyoming bentonite with highly al-  
 678 kaline solutions. Physics and Chemistry of the Earth, Parts A/B/C 32, 275–286.

679 Komine, H., 2004. Simplified evaluation on hydraulic conductivities of sand–bentonite mix-  
 680 ture backfill. Applied Clay Science 26, 13–19.

681 Komine, H., 2010. Predicting hydraulic conductivity of sand–bentonite mixture backfill be-  
 682 fore and after swelling deformation for underground disposal of radioactive wastes. Engi-  
 683 neering Geology 114, 123–134.

684 Krauskopf, K.B., 1956. Dissolution and precipitation of silica at low temperatures. Geo-  
 685 chemica et Cosmochemica Acta 10, 1–26.

686 Lippens, B., Boer, J.H. de, 1965. Studies on pore systems in catalysts V. The t method. Jour-  
 687 nal of Catalysis 4, 319–323.

688 Lloret, A., Villar, M.V., 2007. Advances on the knowledge of the thermo-hydro-mechanical  
 689 behaviour of heavily compacted “FEBEX” bentonite. Physics and Chemistry of the Earth,  
 690 Parts A/B/C 32, 701–715.

691 Mata, C., 2003. Hydraulic behavior of bentonite based mixtures in engineered barriers: The  
692 backfill and plug test at the Aespoe HRL (Sweden). PhD Thesis, Barcelona, 280 pp.

693 Mata, C., Ledesma, A., 2003. Permeability of a bentonite-crushed granite rock mixture using  
694 different experimental techniques. *Géotechnique* 53, 747–758.

695 Michau, N. (Ed.), 2005. ECOCLAY II - Effects of cement on clay barrier performance: Phase  
696 II. Final report, 378 pp.

697 Middelhoff, M., Cuisinier, O., Masrouri, F., Talandier, J., Conil, N., 2020. Combined impact  
698 of selected material properties and environmental conditions on the swelling pressure of  
699 compacted claystone/bentonite mixtures. *Applied Clay Science* 184, 105389.

700 Molinero-Guerra, A., Delage, P., Cui, Y.J., Mokni, N., Tang, A.M., Aïmedieu, P., Bernier, F.,  
701 Bornert, M., 2020. Water-retention properties and microstructure changes of a bentonite  
702 pellet upon wetting/drying; application to radioactive waste disposal. *Géotechnique* 70,  
703 199–209.

704 Norrish, K., 1954. The swelling of montmorillonite. *Discuss. Faraday Soc.* 18, 120.

705 Orsini, L., Remy, J.C., 1976. Utilisation du chlorure de cobaltihexammine pour la détermina-  
706 tion simultanée de la capacité d'échange et des bases échangeables des sols. *Bulletin de*  
707 *l'AFES Science du Sol* 4, 269–275.

708 Pusch, R., Zwahr, H., Gerber, R., Schomburg, J., 2003. Interaction of cement and smectitic  
709 clay—theory and practice. *Applied Clay Science* 23, 203–210.

710 Ramírez, S., Vieillard, P., Bouchet, A., Cassagnabère, A., Meunier, A., Jacquot, E., 2005. Al-  
711 teration of the Callovo–Oxfordian clay from Meuse-Haute Marne underground laboratory  
712 (France) by alkaline solution. I. A XRD and CEC study. *Applied Geochemistry* 20, 89–99.

713 Robinet, J.-C., Tyri, D., Djeran-Maigre, I., 2021. Hydro-mechanical response of crushed ar-  
714 gillite and bentonite mixtures as sealing material. *Engineering Geology* 288, 106140.

715 Romero, E., 2013. A microstructural insight into compacted clayey soils and their hydraulic  
 716 properties. *Engineering Geology* 165, 3–19.

717 Romero, E., Gens, A., Lloret, A., 1999. Water permeability, water retention and microstruc-  
 718 ture of unsaturated compacted Boom clay. *Engineering Geology* 54, 117–127.

719 Seyedi, D.M., Armand, G., Noiret, A., 2017. “Transverse Action” – A model benchmark ex-  
 720 ercise for numerical analysis of the Callovo-Oxfordian claystone hydromechanical re-  
 721 sponse to excavation operations. *Computers and Geotechnics* 85, 287–305.

722 Tang, C.S., Tang, A.M., Cui, Y.J., Delage, P., Shi, B., 2010. The coupled hydro-mechanical  
 723 behaviours of compacted crushed Callovo-Oxfordian argillite. *Journal of Rock Mechanics*  
 724 *and Geotechnical Engineering*, 86–90.

725 Thommes, M., Kaneko, K., Neimark, A.V., Olivier, J.P., Rodriguez-Reinoso, F., Rouquerol,  
 726 J., Sing, K.S., 2015. Physisorption of gases, with special reference to the evaluation of sur-  
 727 face area and pore size distribution (IUPAC Technical Report). *Pure and Applied Chemis-*  
 728 *try* 87, 1051–1069.

729 Tourchi, S., Vaunat, J., Gens, A., Bumbieler, F., Vu, M.-N., Armand, G., 2021. A full-scale in  
 730 situ heating test in Callovo-Oxfordian claystone: observations, analysis and interpretation.  
 731 *Computers and Geotechnics* 133, 104045.

732 Villar, M.V., 2006. Infiltration tests on a granite/bentonite mixture: Influence of water salin-  
 733 ity. *Applied Clay Science* 31, 96–109.

734 Wang, Q., Cui, Y.J., Delage, P., Gatmiri, B., 2012. Experimental study on the swelling behav-  
 735 iour of bentonite/claystone mixture. *Engineering Geology* 124, 59–66.

736 Ye, W.M., Zheng, Z.J., Chen, B., Chen, Y.G., Cui, Y.J., Wang, J., 2014. Effects of pH and  
 737 temperature on the swelling pressure and hydraulic conductivity of compacted GMZ01  
 738 bentonite. *Applied Clay Science* 101, 192–198.

- 739 Yven, B., Sammartino, S., Geraud, Y., Villieras, F., 2007. Mineralogy, texture and porosity of  
 740 Callovo-Oxfordian argillites of the Meuse/ Haute-Marne region eastern Paris Basin). Bul-  
 741 letin de la Societe Geologique de France, 73–90.
- 742 Zeng, Z., Cui, Y.J., Zhang, F., Conil, N., Talandier, J., 2019. Investigation of swelling pres-  
 743 sure of bentonite/claystone mixture in the full range of bentonite fraction. Applied Clay  
 744 Science 178, 105137.
- 745 Zhang, C.-L., 2021. Deformation and water/gas flow properties of claystone/bentonite mix-  
 746 tures. Journal of Rock Mechanics and Geotechnical Engineering 13, 864–874.
- 747 Zhang, C.-L., Kröhn, K.-P., 2019. Sealing behaviour of crushed claystone–bentonite mixtures.  
 748 Geomechanics for Energy and the Environment 17, 90–105.

Table 1: Physical and physico-chemical characteristics of COX<sub>c</sub>, MX80-bentonite and the mixture

	Initial water content	Liquid limit	Plastic limit	Specific gravity	Cation ex- change cap.	Ext. specific surface area
	$w_{ini}$	$LL$	$PL$	$G_s$	CEC	SSA
	[%]	[%]	[%]	[-]	[meq/100g]	[m <sup>2</sup> /g]
COX <sub>c</sub>	5.4	37.3	24.9	2.68	22	37
Mixture	6.4	112.5	34.7	2.64	39	32

751 Table 2: Major compounds and properties of site solution and alkaline solution (Middelhoff et al., 2020)

	Na <sup>+</sup>	K <sup>+</sup>	Ca <sup>2+</sup>	Mg <sup>2+</sup>	Cl <sup>-</sup>	HCO <sub>3</sub> <sup>-</sup>	SO <sub>4</sub> <sup>2-</sup>	OH <sup>-</sup>	IS	pH
	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[mmol/L]	[-]
SS	44.8	0.5	5.3	8.5	35.3	1.5	1.8	-	105	7.8
AS	22.1	8.0	19.0	-	30.1	-	-	38.1	87	12.5

SS: Site solution; AS: Alkaline solution; IS: Ionic strength

752

753 Table 3: Initial characteristics of employed samples

Material	Initial dry density	Initial water content	Initial void ratio	Initial poro- sity	Initial degree of saturation	Initial suc- tion
	$\rho_{d, ini}$	$w_{ini}$	$e_{ini}$	$n_{ini}$	$S_r$	$s$
	[Mg/m <sup>3</sup> ]	[%]	[-]	[-]	[-]	[MPa]
COX <sub>c</sub>	1.95	13.2	0.37	0.27	0.96	1.5
Mixture	1.72	18.2	0.53	0.35	0.91	2.7

755 Table 4: Experiment program including hydraulic conductivity experiments, microstructural and textural analysis

Sample ID.	C-I	C-W	C-SS	C-AS	M-I	M-W	M-SS	M-AS
Material	COX <sub>c</sub>	COX <sub>c</sub>	COX <sub>c</sub>	COX <sub>c</sub>	Mixture	Mixture	Mixture	Mixture
Solution	-	Water	SS	AS	-	Water	SS	AS
Constant head ex- periment	-	✓	✓	✓	-	✓	✓	✓
Mercury Intrusion Porosimetry (MIP)	✓	-	-	-	✓	-	✓	✓
N <sub>2</sub> -gas sorption	✓	-	-	-	✓	-	✓	✓

✓: Experiment performed

757

758

Table 5: Absolute and relative values of microstructural characteristics of top and bottom parts of C-I, M-I, M-SS and M-AS

Sample-ID.	Absolute					Relative		
	$n^*$	$e_{tot}$	$e_M$	$e_m$	$e_\mu$	$e_M/e_{tot}$	$e_m/e_{tot}$	$e_\mu/e_{tot}$
	[-]	[-]	[-]	[-]	[-]	[-]	[-]	[-]
C-I (T)	0.28	0.388	0.225	0.111	0.052	0.58	0.29	0.13
C-I (B)	0.27	0.368	0.209	0.112	0.047	0.57	0.30	0.13
M-I (T)	0.34	0.501	0.287	0.146	0.068	0.57	0.29	0.14
M-I (B)	0.37	0.591	0.386	0.143	0.062	0.65	0.24	0.10
M-SS (T)	0.30	0.435	0.254	0.158	0.023	0.58	0.36	0.05
M-SS (B)	0.32	0.470	0.261	0.150	0.059	0.56	0.32	0.13
M-AS (T)	0.29	0.399	0.236	0.150	0.013	0.59	0.38	0.03
M-AS (B)	0.29	0.406	0.238	0.157	0.011	0.59	0.39	0.03

\*: measured in MIP-experiments

Sample	Samp. part	Height	BET			t-plot	
		h [mm]	$V_m^a$ [cm <sup>3</sup> /g]	$C^b$ [-]	$S_{Tot}^c$ [m <sup>2</sup> /g]	$V_{\mu pores}^d$ [cm <sup>3</sup> /g]	$S_{Tot}^c$ [m <sup>2</sup> /g]
C-I	Bottom	3	8.39	274	37	0.0029	37
	Middle	15	8.39	274	37	0.0029	37
	Top	27	8.39	274	37	0.0029	37
M-I	Bottom	3	7.43	431	32	0.0043	31
	Middle	15	7.43	431	32	0.0043	31
	Top	27	7.43	431	32	0.0043	31
M-SS	Bottom	3	7.44	331	33	0.0032	32
	Middle	15	7.37	875	32	0.0032	32
	Top	27	7.00	964	31	0.0052	29
M-AS	Bottom	3	7.36	435	32	0.0039	31
	Middle	15	7.23	697	32	0.0039	31
	Top	27	7.00	762	31	0.0046	29

<sup>a</sup>: Associated monolayer capacity; <sup>b</sup>: BET energy constant; <sup>c</sup>: Total specific surface area; <sup>d</sup>: Equivalent specific volume of micropores

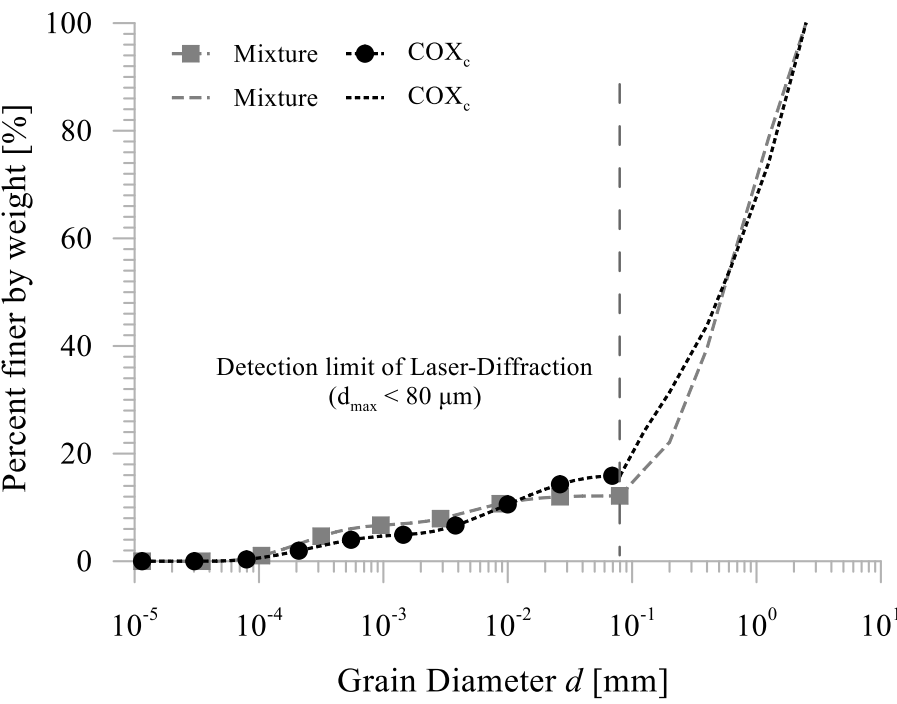
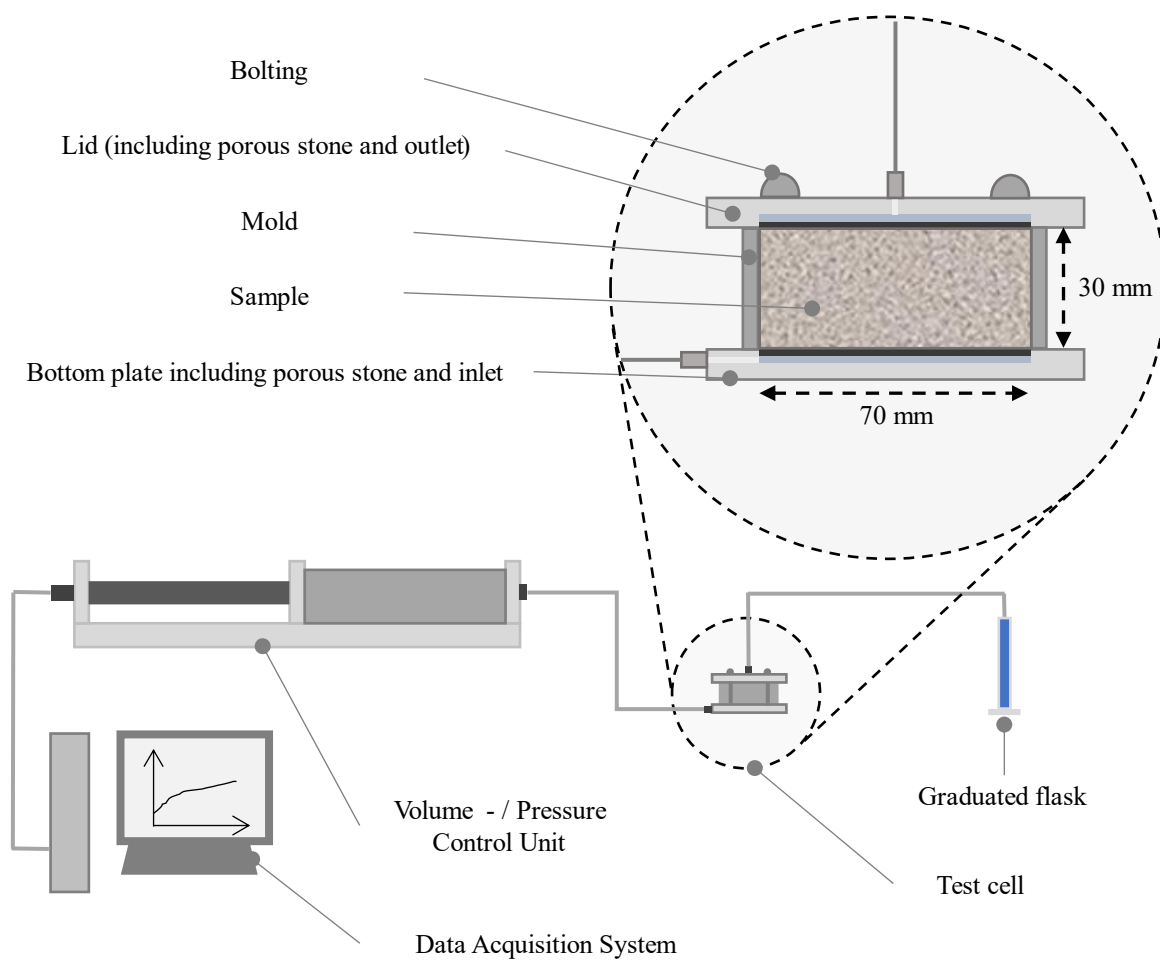


Figure 1: Grain size distribution curves of COX<sub>c</sub> and the mixture obtained by dry sieving and laser diffractometry (Middelhoff et al., 2020)



767

768 Figure 2: Setup of hydraulic conductivity experiments

769

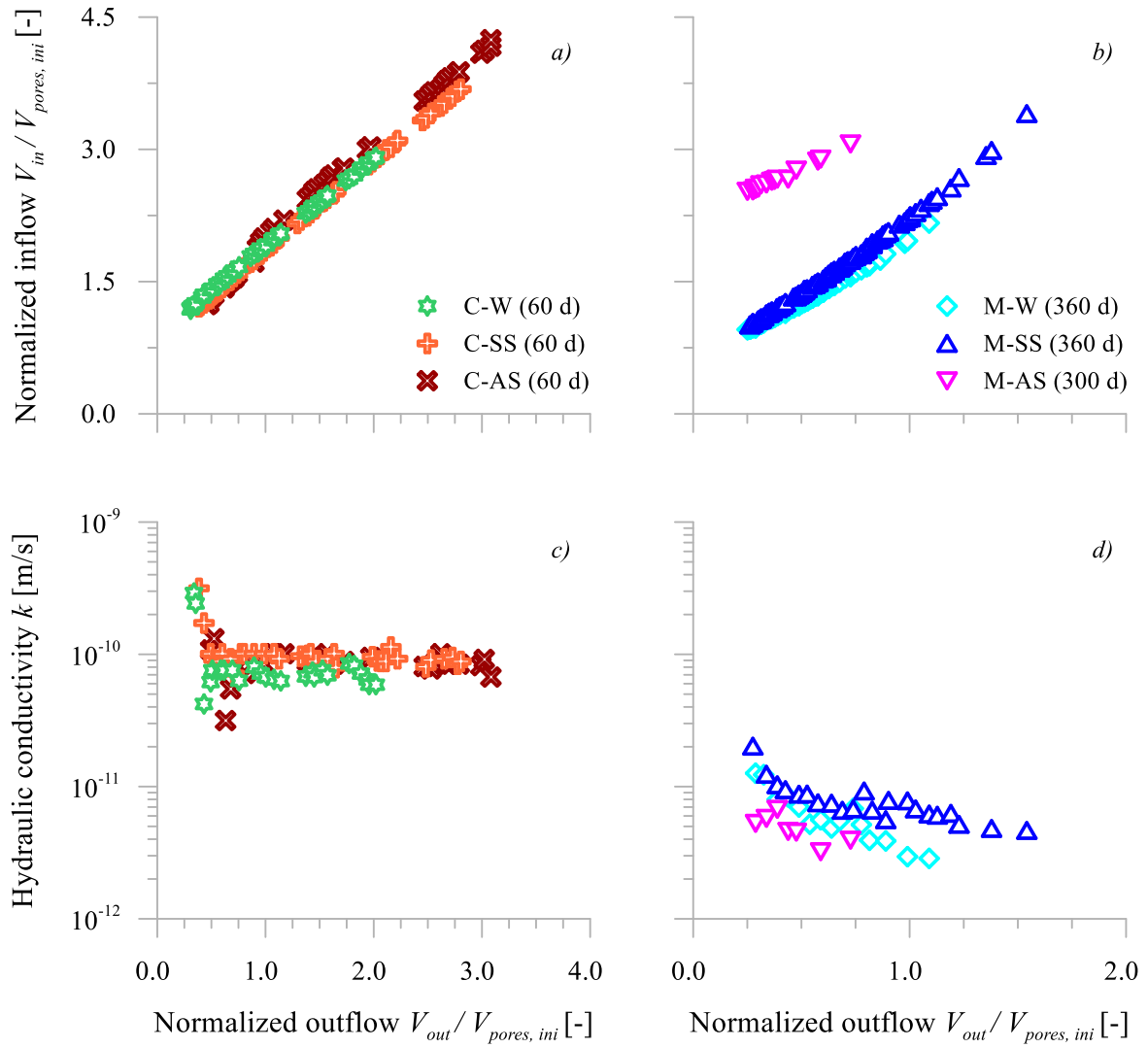


Figure 3: Evolution of the normalized inflow and the hydraulic conductivity of a, c) COX<sub>c</sub> ( $\rho_{d, ini} = 1.95 \text{ Mg/m}^3$ ) and b, d) the mixture ( $\rho_{d, ini} = 1.72 \text{ Mg/m}^3$ ) as function of the normalized outflow (saturated with water (-W), site solution (-SS) and alkaline solution (-AS))

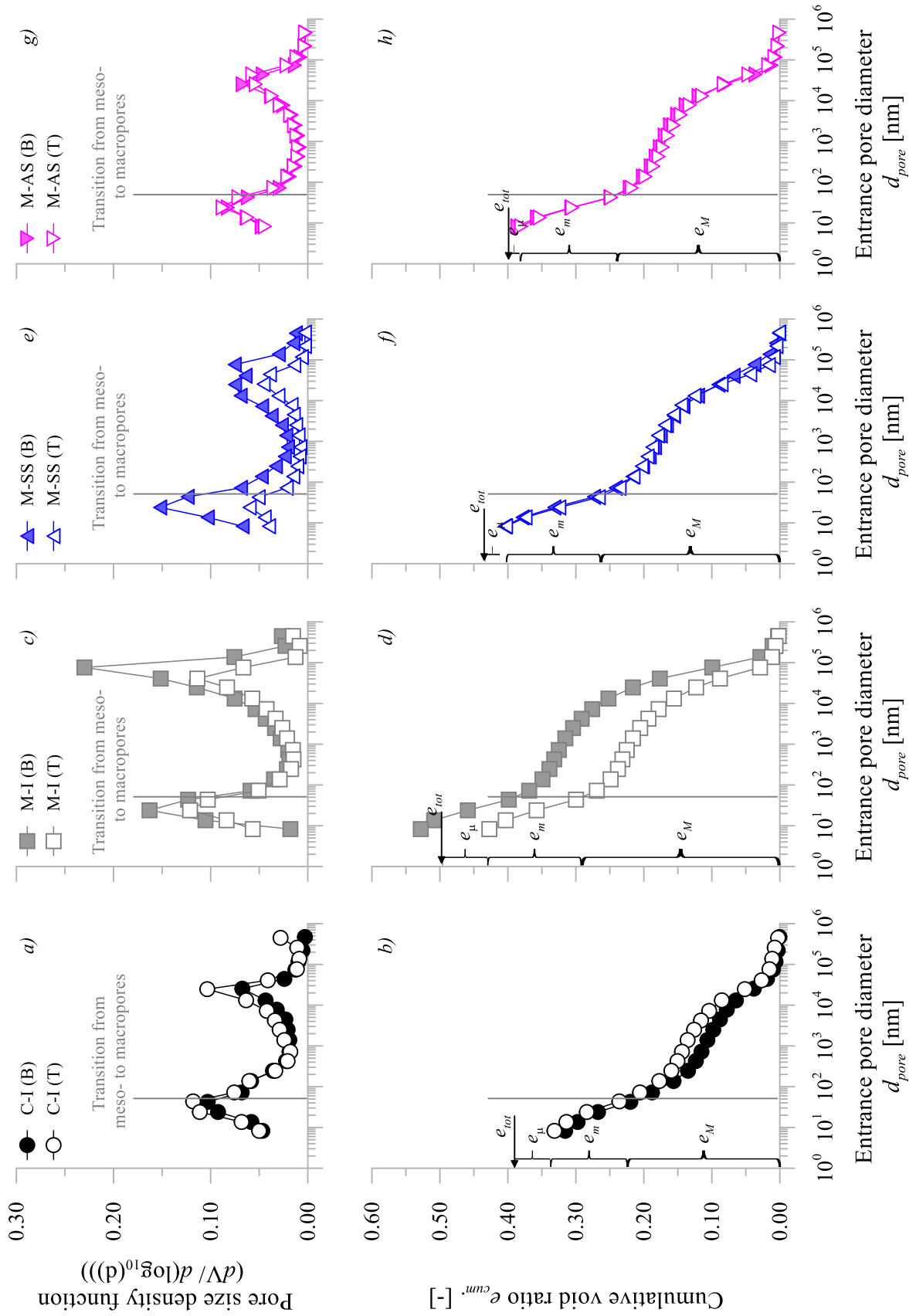
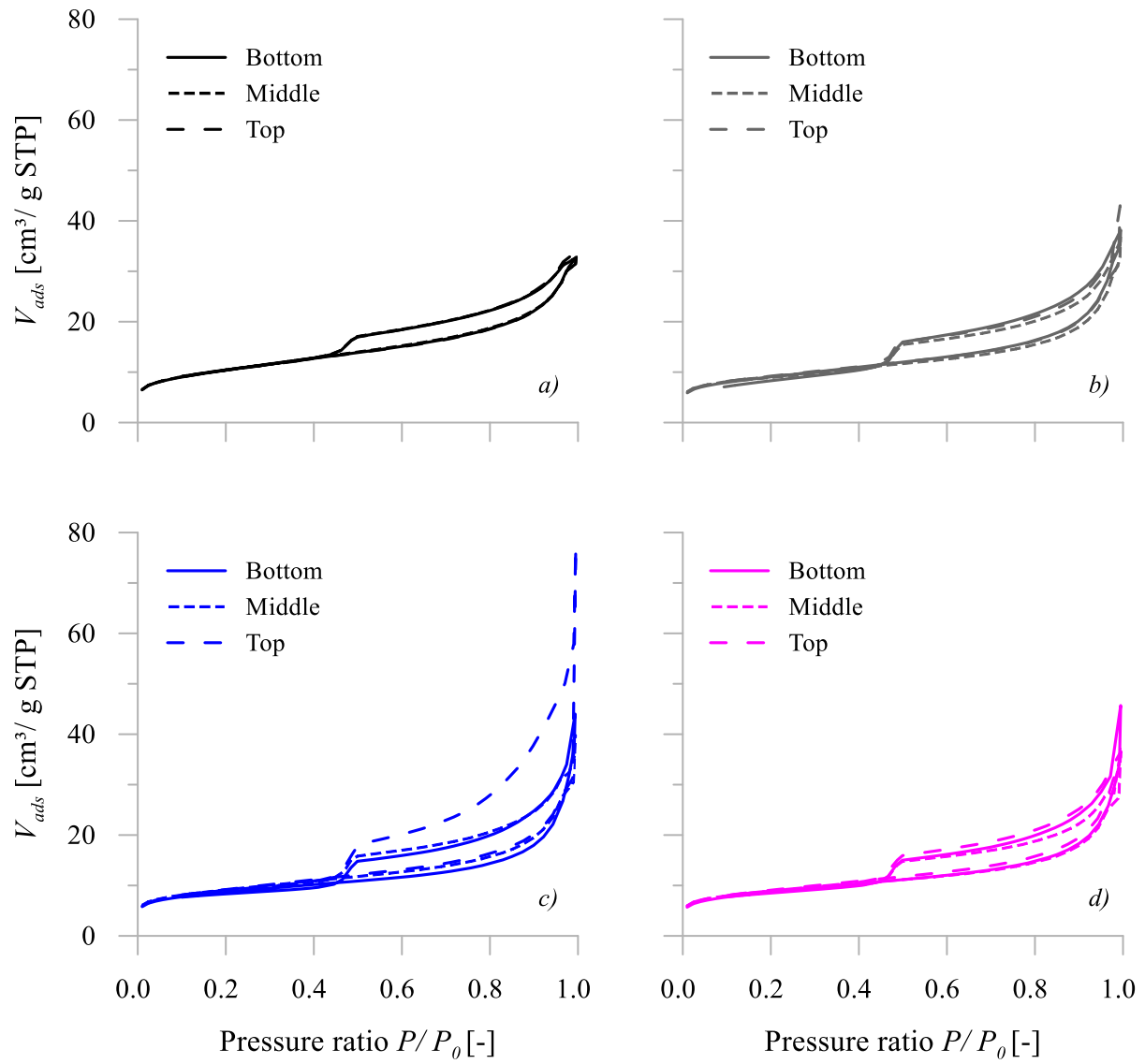


Figure 4: Results of mercury intrusion porosimetry (MIP) experiments conducted on the top and bottom parts of samples C-I, M-I, M-SS and M-AS



778

779 Figure 5: Isotherms\* obtained by means of N<sub>2</sub>-sorption experiments conducted on the top (27 mm), middle  
 780 (15 mm) and bottom parts (3 mm) of samples C-I, M-I, M-SS and M-AS (\* Pressure ratio versus volume of gas  
 781 adsorbed ( $V_{ads}$ ) at standard pressure and temperature (SPT))